

# HOLMSTROM HEIGHTS SUBDIVISION & STREETS A PORTION OF PARCEL 1 OF LAND PARTITION NO. 2006-02 AND OTHER LANDS SITUATED IN THE NE1/4, SE1/4 AND NW1/4 SECTION 23, T.13S., R.31E. W.M., JOHN DAY, GRANT COUNTY, OREGON END OF CHAROLAIS DR. O.B. HOLMSTROM ROAD N89"59'55"E OINT BEING 76.97 FT. ORTH AND 3595.73 FT. EAST FROM THE W1/4 COR. SEC. 23 HOLMSTROM ROAD HOLMSTROM PARCEL 1, L.P. 2006-02 HOLMSTROM ROAD RIGHT OF WAY DESCRIPTION A Right of way for street purposes, situated in the W1/2E1/2 Section 23, T.13S., R.31E., W.M., and being a portion of Parcel 1 of Land Partition No. 2006-02, according to the Plat thereof, on file and of record in the office of the County Clerk of said County and State, filed February 23, 2006, said right of way being described as follows: Beginning at the southeasterly terminus of the centerline of Charolais Drive, said point also being 76.97 feet North and 3595.73 feet East from the W1/4 Corner of said Section 23, thence N.89°59'55"E., 35.00 feet to a point of the easterly line of the herein described right of way; thence along said easterly right of way line as follows: South, 331.55 feet; 70.67 feet along the arc of a 65.00 foot radius curve left (the long chord of which bears S.31°08'55'E., 67.24 feet); 293.57 feet along the arc of a 135.00 foot radius reverse curve right (the long chord of which bears South, 239.05 feet); 70.67 feet along the arc of a 65.00 foot radius reverse curve left (the long chord of which bears S.31°08'55"W., 67.24 feet); SCHOOL DISTRICT NO. 3 South, 781.03 feet; BK. 122, PG. 41 LINE thence, leaving said east right of way line, West, 60 feet, more or less, to the west line of Parcel 1 of said Land Partition NW1/4 Plat No. 2006-02; SE1/4 **LIMITS** thence along the west line of said Parcel 1, North, 1466.73 feet to a point S.89°59'55". W. from the point of beginning; thence N.89°59'55"E., 25.00 feet to the point of beginning. CITY EXCEPTING THEREFROM the city of John Day's Well site No. 3, more particularly described as follows: Beginning at a point that is S.74°59'W., 1703.83 feet from the E1/4 Corner of Section 23; thence North, 35.59 feet; thence West, 100.00 feet; thence South, 100.00 feet; thence East, 100.00 feet; HOLMSTROM ROAD SOUTH 781.03 thence North, 63.41 feet to the point of beginning. SEVENTH STREET RIGHT OF WAY DESCRIPTION A Right of way for street purposes, situated in the SW1/4SE1/4 Section 23, T.13S., R.31E., W.M., and being a portion that certain tract of land described in deed record book 122, page 41, deed records of Grant County, Oregon, said right of way being described as follows: **HOLMSTROM** Beginning at a point on the west boundary of Parcel 1 of Land Partition No. 2006-02, according to the Plat thereof, on PARCEL 1, L.P. 2006-02 R/W CURVE DATA A=32°39'58" R=270.00 L=153.93 LC=151.86 S80°55'04"E file and of record in the office of the County Clerk of said County and State, filed February 23, 2006, said point being 1389.76 feet South and 3570.67 feet East from the W1/4 Corner of said Section 23, thence S.82°45'00'W., 203.84 feet; thence 182.44 feet along the arc of a 320.00 foot radius curve right (the long chord of which bears N.80°55'01"W., 179.98 feet); S80.55'01"E R/W CURVE DATA thence N.25°24'58"E., 50.00 feet; A=32°39'58" R=320.00 L=182.44 LC=179.98 25 | 35 thence 153.93 feet along the arc of a 270.00 foot non-tangent curve left (the long chord of which bears S.80°55'01"E., 151.86 feet); 180°55'01"W N82°45'00"E 183.84 thence N.82°45'00"E., 183.84 feet; SW1/4 SEVENTH STREET SE1/4 thence 43.33 feet along the arc of a 30.00 foot radius curve left (the long chord of which bears N.41°22'30'E., 39.66 S82°45'00"W 203.84 feet) to a point on the west line of Parcel 1 of said Land Partition Plat No. 2006-02; 60.00 thence South, 76.84 to the point of beginning. EXISTING PRIVATE 30 FT. POINT BEING 1389.76 FT. SOUTH AND 3570.67 FT. EAST FROM THE ACCESS & UTILITY EASEMENT BK. 122, PAGE 41 ROOKSTOOL W1/4 COR. SEC. 23 NOTE: THE BEARINGS, DISTANCES AND AREAS SHOWN HEREON MAY CHANGE DEROSIER UPON THE EXECUTION OF A SURVEY BENCHMARK LAND SURVEYING 217 N. CANYON BLVD. JOHN DAY, OREGON **REGISTERED** $541-575-1251 \sim benchmarkls.com$ **PROFESSIONAL** LAND SURVEYOR PRELIMINARY SUBDIVISION PLAT OF HOLMSTROM HEIGHTS & STREETS A PORTION OF PARCEL 1 OF LAND PARTITION NO. 2006–02 SITUATED IN THE NE1/4 AND NW1/4 SECTION 23, T.13S., R.31E. W.M., JOHN DAY, GRANT COUNTY, OREGON OREGON MICHAEL C. SPRINGER SURVEYED FOR FRANCIS HOLMSTROM & THE CITY OF JOHN DAY #70918 MCS 3/4/2022 SURVEYED BY EXPIRES: 6/30/2022

SCALE: 1"=100'

DRAWN BY: MCS

SHEET 2 OF 2

PRELIMINARY PLAT OF

#### Streets:

- 1. New street sections are to be cleared of all surface vegetation, stumps, and other miscellaneous structures or materials. Grub improvement areas to remove all buried vegetative matter and debris to a depth of 8" below subgrade. Legally dispose
- 2. Immediately following fine grading operations proof roll subgrade areas to achieve 95% of maximum density for a 12" depth per AASHTO T-180 test method. Embankments or fills are to be constructed in 6" moximum lifts, with each lift being compacted to 95% maximum density prior to proceeding with the next lift. Areas to receive fill are to be inspected by City
- 3. Aggregate base rock shall be 1"-0" crushed rock as per Oregon State Highway Division specifications. Aggregate base is to be compacted in 6" maximum lifts to 95% of maximum density per AASHTO T-180 test method. Subgrade is to be inspected by the City and the Engineer prior to placement of the base rock. Base is to be inspected prior to placement of asphalt. Sub-base and base are to be proof rolled during the Engineer's/Inspector's inspection of the subgrade and base
- 4. The first lift of asphalt concrete is to be Class 'B' A.C. as per City of John Day specifications. The second lift, shall be Class 'C' asphalt concrete as per City of John Day specifications. Pave only during dry weather and when the temperature is 40° or warmer. Asphalt concrete shall be compacted to 91% of Rice density.
- 5. Construct curb and gutter, per detail, using Class 'A' 3300 psi concrete with maximum 1-1/2" aggregate size. Expansion joints shall be installed at 45' maximum on centers, contraction joints at 15' maximum on centers. Three and one—half inch weepholes are to be installed 5 feet from the property line.
- 6. All materials, installation, tests, and inspections are to be in strict accordance with City of John Day Public Works
- 7. Install street barricades at the end of temporary deadends.

- 1. Twelve inch and larger storm drain pipe shall be Class 5 reinforced concrete pipe conforming to ASTM C14 & C76 or HDPE pipe conforming to AASHTO M-294s (with watertight gaskets), unless otherwise specifically noted on the plan or profile. Rubber joints for concrete pipe are required only where specifically noted on the plans. Six inch storm drain pipe shall conform to HDPE smooth interior, corrugated exterior pipe.
- Granular backfill is to be compacted to 95% maximum density per AASHTO T180 test method and native material shall be compacted to 85% of in place density of surrounding soil. All trenches within existing and new public street right—of—ways shall be backfilled with acceptable select imported granular soils and compacted to a minimum of 95% relative density (AASHTO T-180) for the upper 36" of the trench. Below 36" the compaction shall be to a minimum 90% relative density.
- 3. If during the course of installing the underground utilities drain tiles are intercepted the tiles shall be piped directly into the storm system after approval of the inspecto
- 4 .All materials, installation, tests, and inspections to be made in strict accordance with City of John Day Standard

#### Sanitary Sewer:

- 1. Pipe shall be PVC sewer pipe conforming to ASTM D-3034 SDR 35. Minimum stiffness shall be 46 psi and joint type shall be elastomeric gasket conforming to ASTM D-3212.
- 2. Manhole base shall be poured in place concrete base with a minimum compressive strength of 3000 psi, or precast base (see detail sheet). Manhole risers and tops shall be precast sections with a minimum compressive strength of 4000 psi. Tops shall be eccentric cones except where insufficient headroom requires flat tops. Inverts shall be constructed to provide smooth manhole by means of an elastomeric gasket, an approved waterstop or flexible sleeve. Cement grout for connecting PVC sewer pipe to manhole will not be permitted.
- 3. Cleanout pipe, fittings and joints shall be the same specifications as for pipe. Castings are as shown on detail and shall conform to ASTM A48 (Grade 30).
- 4. Granular backfill shall be compacted to 95% maximum dry density per AASHTO T-180 test method and native material shall be compacted to 90% maximum dry density per AASHTO T-180. Native material allowed in roadways or under sidewalk
- 5. PVC service laterals shall be 4" and 6" (per construction plans) pipe conforming to the same specifications as the sewer mains. Service laterals shall be installed to a point beyond the line of the sewer or utility easement as shown on the plan. The service lateral shall be plugged with a 4" or 6" rubber ring plug and the location of the lateral end shall be marked with a 2" x 4" board. The sanitary lateral shall have an identify tape laid with the lateral and tied to the 2"x4"
- 6. Sanitary sewer pipe and appurtenances shall be tested for leakage in accordance with APWA Division III requirements. Leakage tests will include required APWA air pressure test for sewer lines and required APWA vacuum test of the manholes. All sewer lines shall be tested for deflection with a mandrel equal to 95% of the pipe size being tested for deflection with a mandrel equal to 95% of the pipe size being tested per APWA Division III, Section 303.3.10. All tests shall be witnessed
- 7. All materials, installation, tests, and inspections are to be made in strict accordance with APWA's Standard Specifications for Public Works Construction

Erosion and Sediment Control Requirements:

- 1. The intent of the requirement is to prevent siltation from reaching storm drain systems and drainage ways. The erosion and sediment control (ESC) facilities shown on this plan are the minimum requirements for anticipated site conditions. During the construction period, these ESC facilities shall be upgraded as needed for unexpected storm events and to ensure that sediment laden water does not leave the site.
- 2. The following controls and practices are required:
- a) Each site shall have graveled or paved entrances, exits and parking areas, prior to beginning any other work, to reduce the tracking of sediment onto public or private roads.
- All uppayed roads located on-site shall be graveled. Other effective erosion and sediment control measures either on the road or down gradient may be used in place of graveling.
- When trucking saturated soils from the site, either water-tight trucks shall be used or loads shall be drained on-site until dripping has been reduced to minimize spillage on roads.
- Concrete trucks being washed out onsite shall be parked in a location that will prevent all wash water from entering the storm drain system without proper filtration. Concrete remnants and residue shall be properly disposed of.
- 3. Additional controls and practices shall be developed that are appropriate for the site. At a minimum the following shall be
- Whenever practicable, clearing and grading shall be done in a phased manner to prevent exposed inactive areas from becoming a
- In developing vegetative erosion control practices, at a minimum the following shall be considered; temporary seeding, permanent seeding, mulching, sod stabilization, vegetative buffer strips, and protection of trees with protective construction fences.
- The following shall be considered for the protection of exposed areas and the prevention of soil from being eroded by storm water; mulching with straw or other vegetation, use of erosion control blankets, and application of soil tackifiers.
- d) The following shall be considered for the diversion of flows from exposed soil, store flows to allow for sedimentation, filter flows, or otherwise reduce soil laden runoff; use of silt fences, earth dikes, brush barriers, drainage swales, check dams, subsurface drains, pipe slope drains, rock outlet protection, sediment traps, and temporary or permanent sedimentation basins. All temporary sediment control practices shall not be removed until permanent vegetation or other cover of exposed areas is
- e) The following shall be considered to prevent the stockpiles from becoming a source of erosion; diversion of uncontaminated flows around stockpiles, use of cover over stockpiles, and installation of silt fences around stockpiles.
- 4. The following maintenance activities shall be implemented:
- Significant amounts of sediment that leave the site shall be cleaned up within 24 hours and placed back on the site or properly disposed. Any in-stream clean up of sediment shall be performed according to Oregon Division of State Lands' required
- Under no conditions shall sediment intentionally be washed into storm sewer or drainage way unless it is captured by a BMP before entering receiving waters.
- For a filter fence, the trapped sediment shall be removed when it reaches one third of the above ground fence height.
- For catch basin protection, cleaning must occur when design capacity has been reduced by fifty percent.
- For a sediment basin, removal of trapped sediments shall occur when design capacity has been reduced by fifty percent. All erosion and sediment controls not in the direct path of work shall be installed before any land disturbance.
- If fertilizers are used to establish vegetation, the application rates shall follow manufacturer's guidelines and the application shall be done in such a way to minimize nutrient—laden runoff to receiving waters.
- If construction activities cease for thirty (30) days or more, the entire site must be stabilized, using vegetation of a heavy mulch layer, temporary seeding, or another method that does not require germination to control erosion.
- Any use of toxic or other hazardous materials shall include proper storage, application, and disposal.
- The permittee shall manage abandoned hazardous wastes, used oils, contaminated soils or other toxic substances discovered during construction activities in a manner approved by the Department of Environmental Quality.

### Erosion and Sediment Control Inspection Requirements:

- All sites 5 acres and greater shall have a person with knowledge and experience in construction storm water controls and management practices conduct all inspections. The inspector shall keep a written record of each inspection.
- 2. Active Sites: Frequency of inspections shall be daily during storm water runoff or snowmelt runoff and at least once every seven (7) calendar days and within 24 hours after any storm event of greater than 0.5 inches of rain per 24-hour period.
- 3. Inactive Sites: During inactive periods of greater that seven (7) consecutive calendar days, inspections shall only be required once every two (2) weeks. Prior to discontinuing activities at the site, any exposed area shall be stabilized to prevent erosion. Stabilization may occur by applying appropriate cover (mulch, erosion control blanket, soil tackifier, etc.) or establishing adequate

#### Structurai Fill Notes:

- 1. All miscellaneous materials and the organic layer under the fill area shall be stripped or removed. All stumps in the fill area must be removed in their entirety.
- 2. The contractor shall follow the procedures identified by the Geotechnical Engineer for constructing structural fills.
- 3. General site preparations should include the reconstruction of miscellaneous un-documented fills by removing a minimum of 4 feet of material, and replacement to structural fill standards. Where concentrated boulder backfills are present, a portion of the boulders should be removed and the remainder mixed with on—site sandy soils prior to replacement. The approximate extent and locations of un-documented fills evidenced are provided on the Site Plan, Figure 1 of the GERED PROFESS geotechnical investigation. Additional greas may be present or evidenced during utility trenching.

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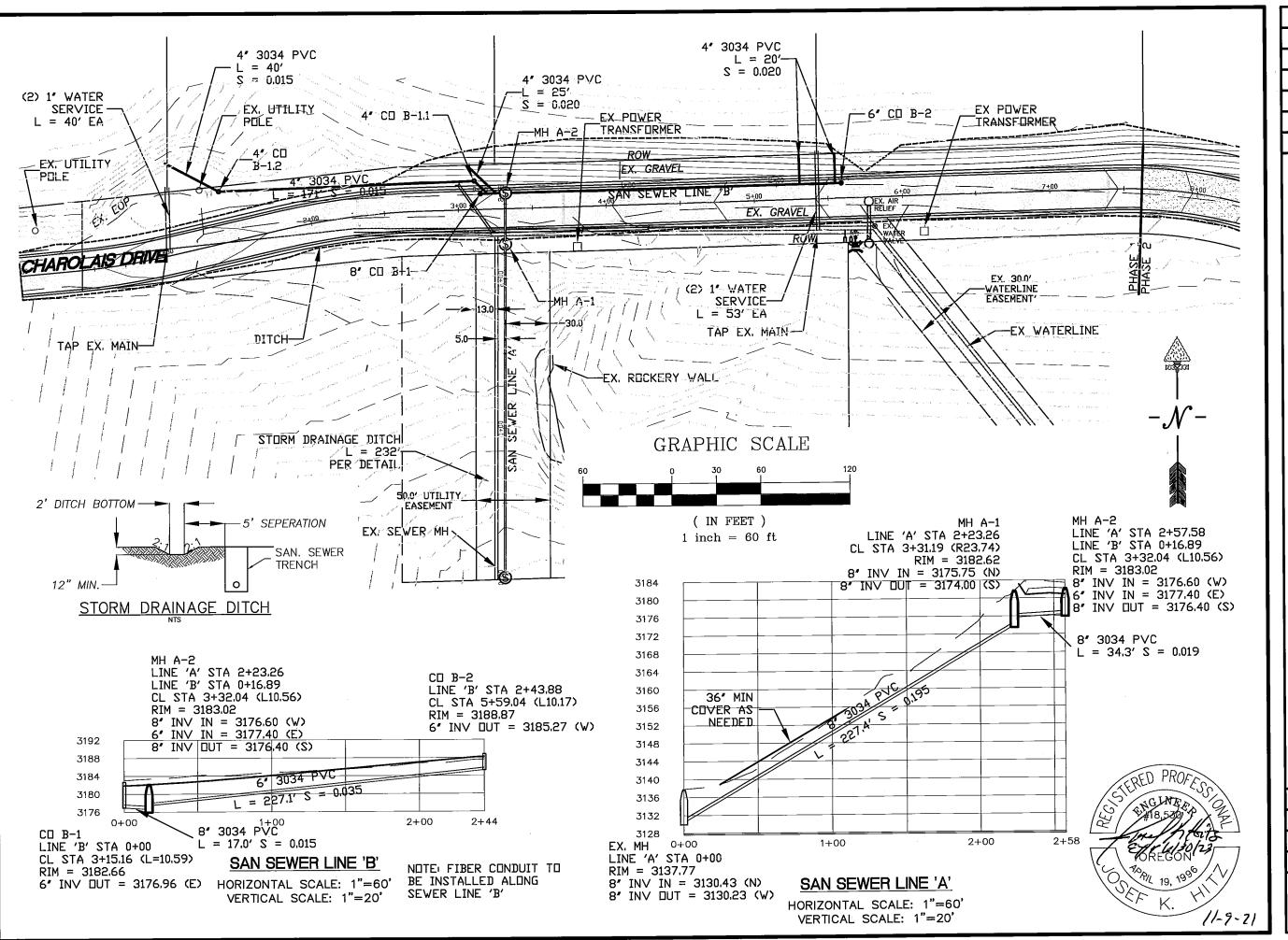
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WATER AND SANITARY 2021 EAST END SEWER PLAN AND CHAROLAIS DRIVE EXT. PROFILE ENGINEERING

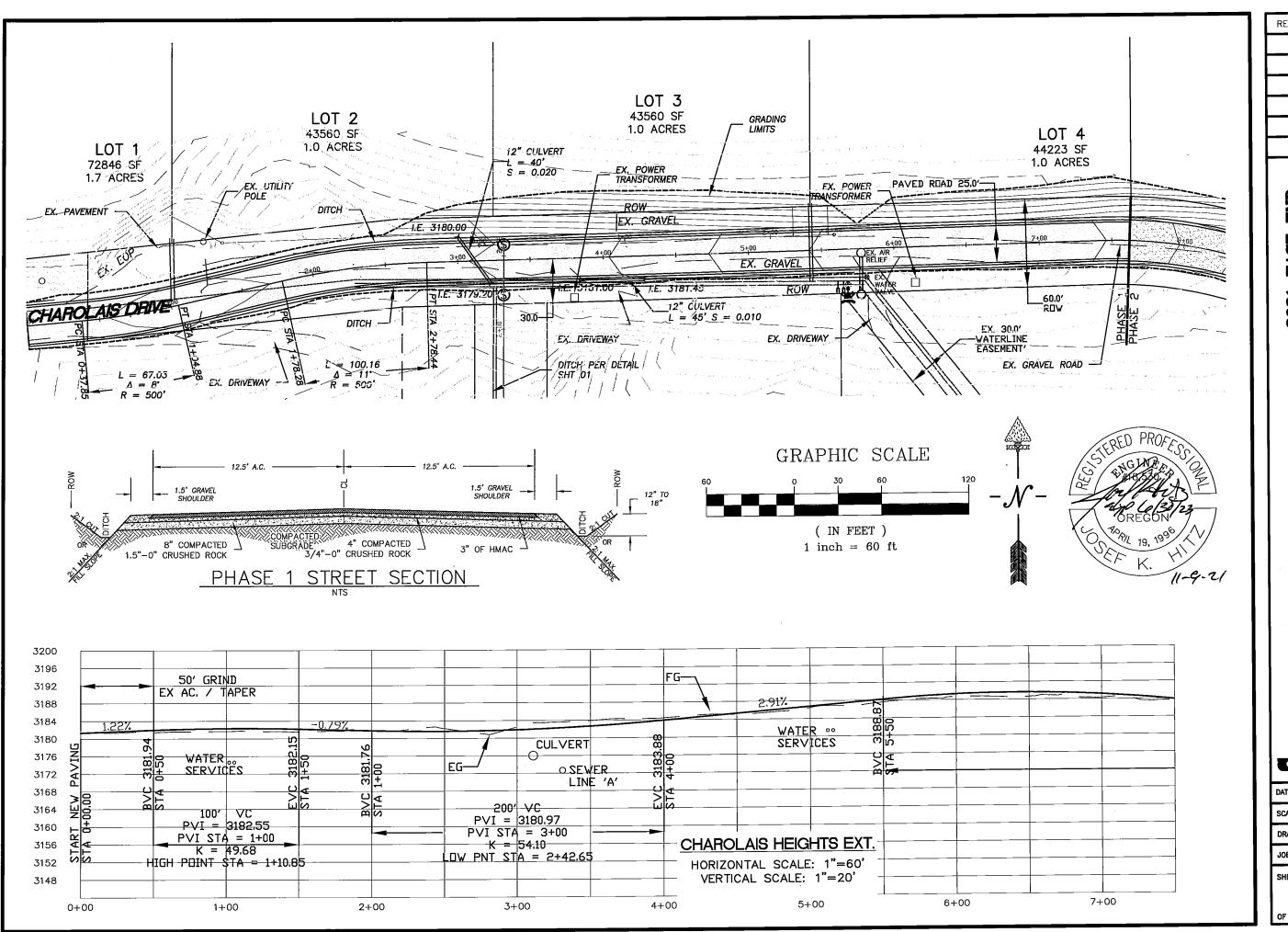
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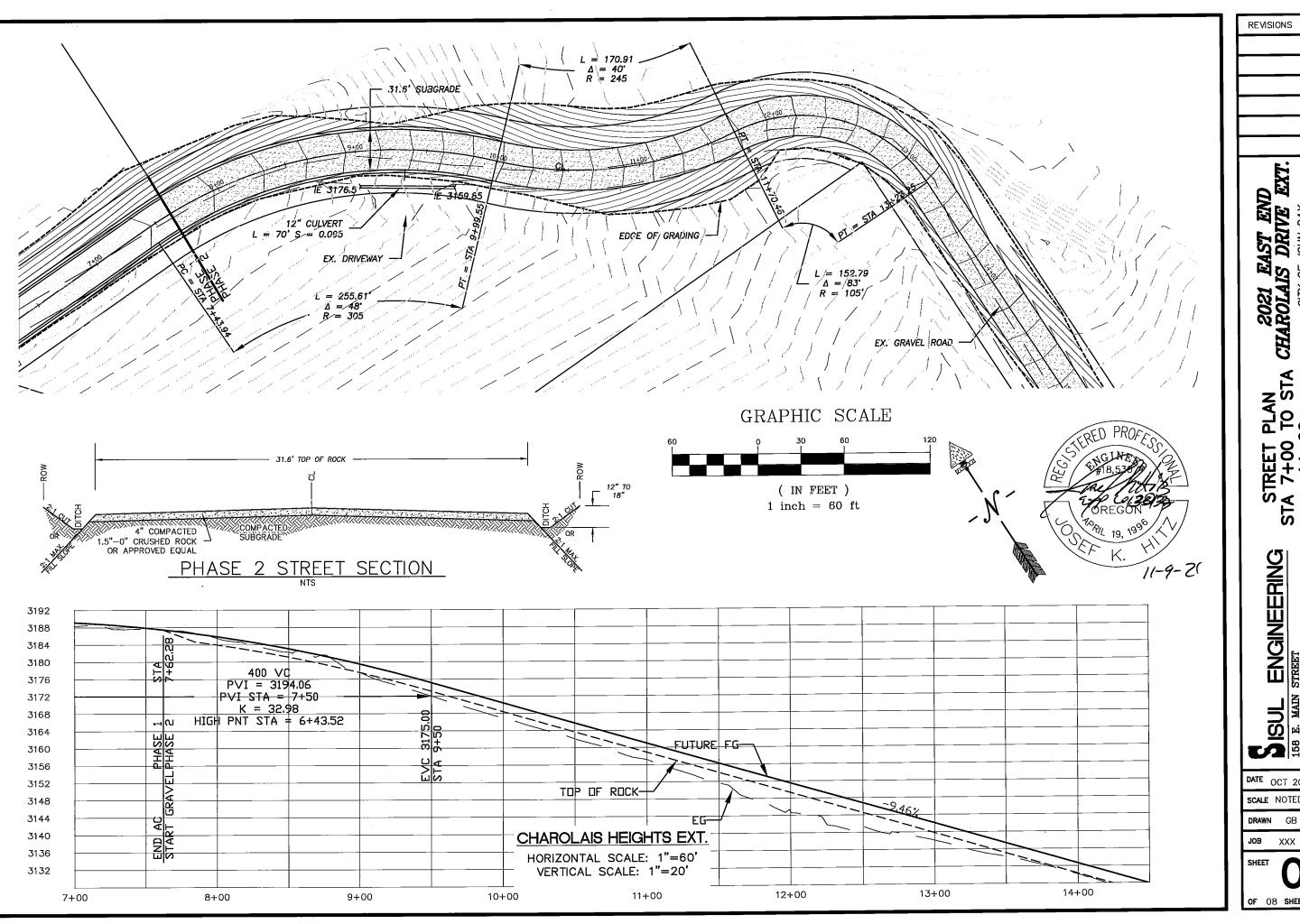
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JOHN DAY, OREGON
(541) 575-8777

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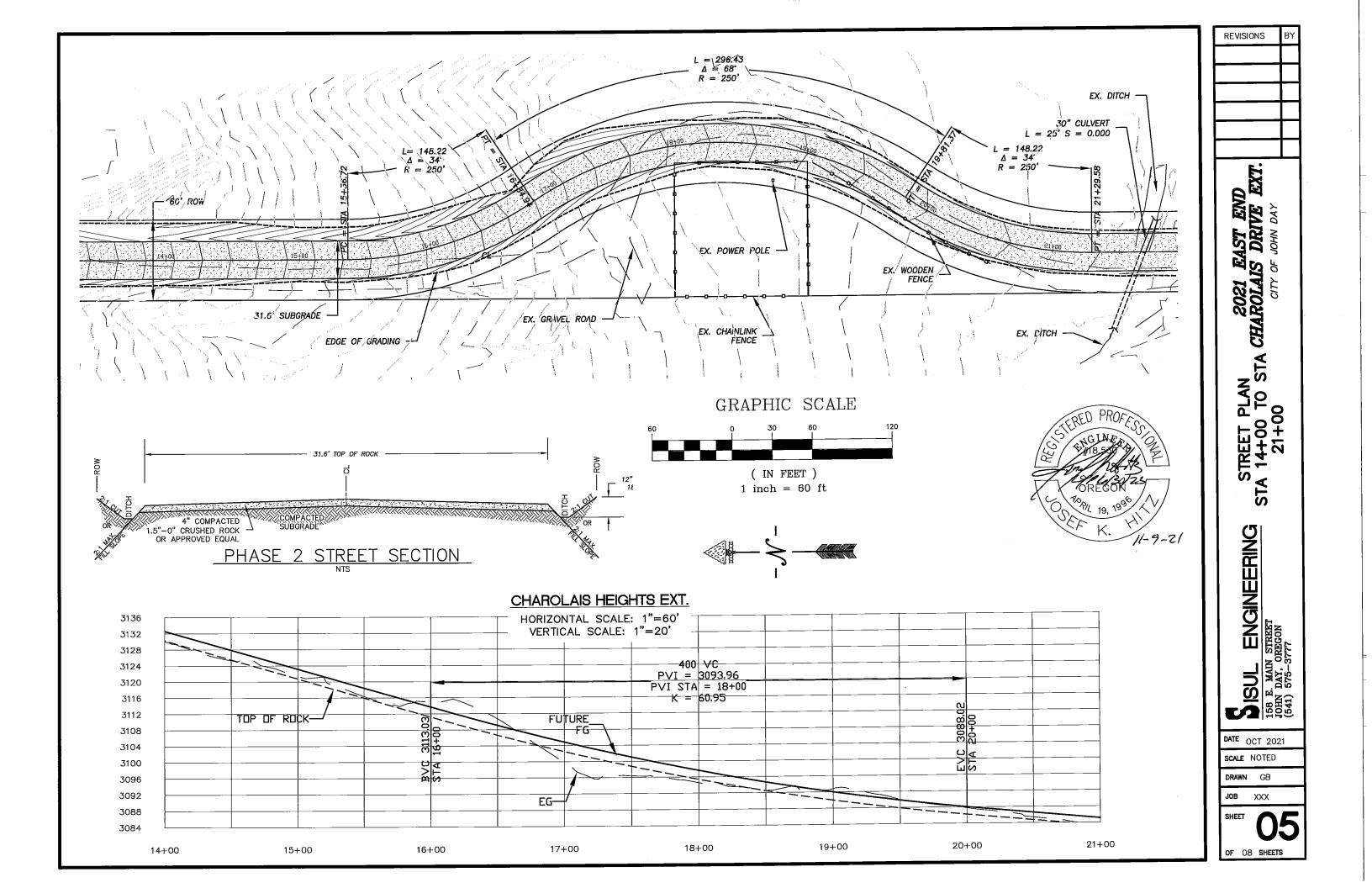


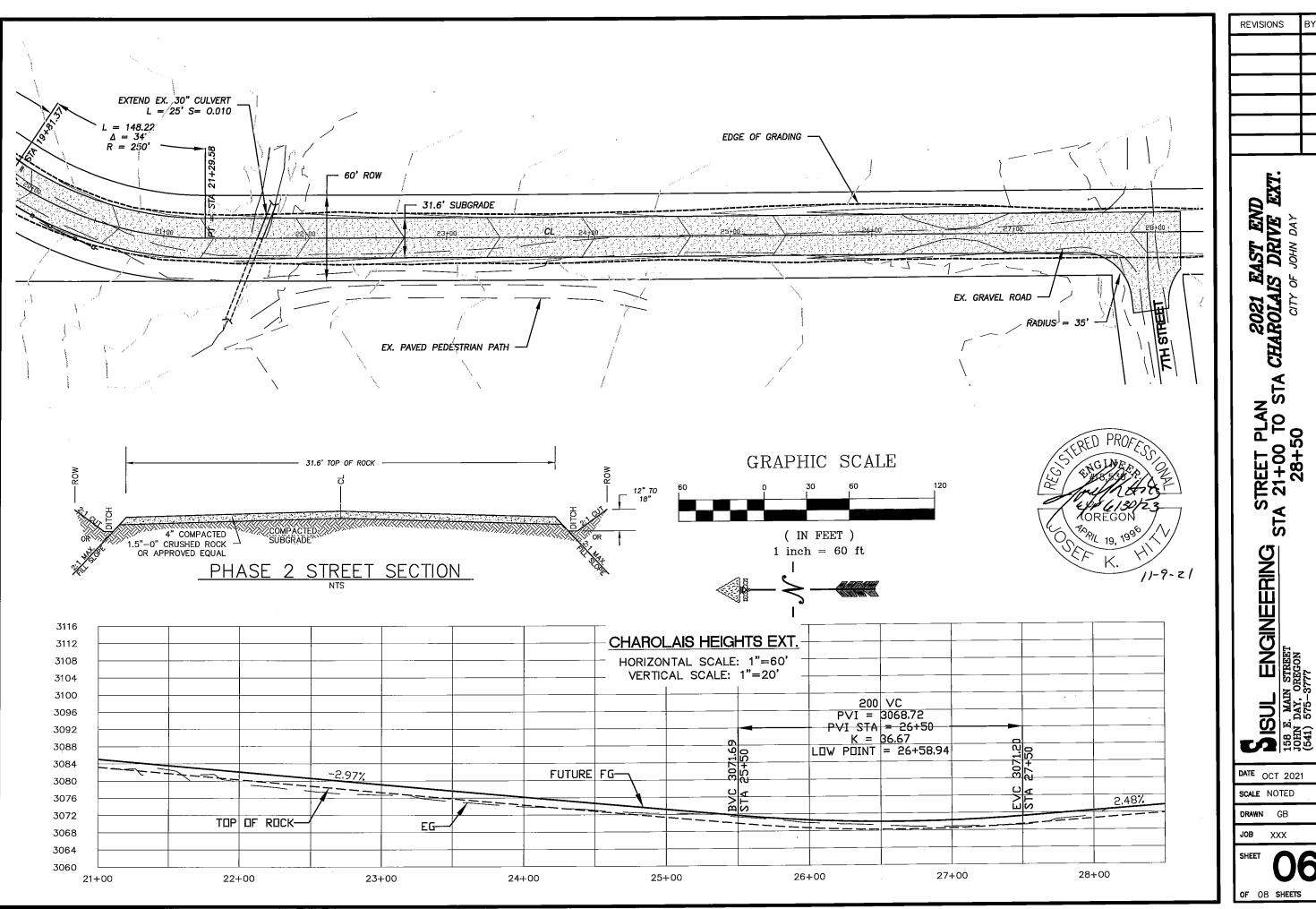
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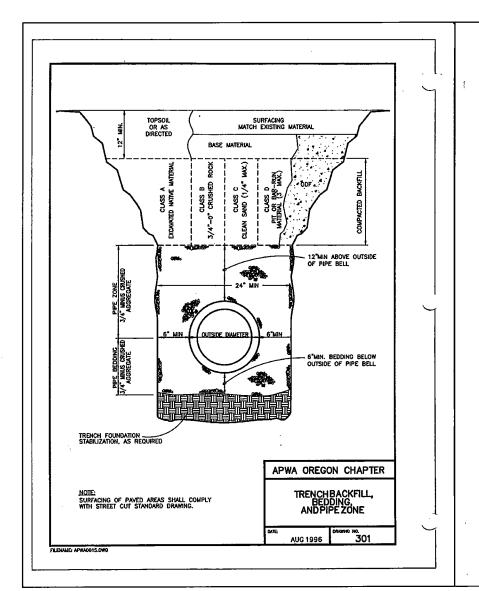


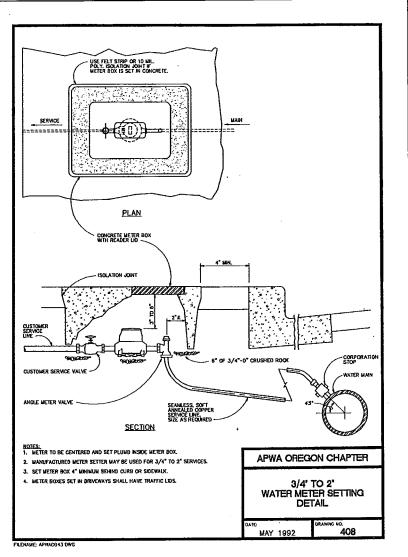
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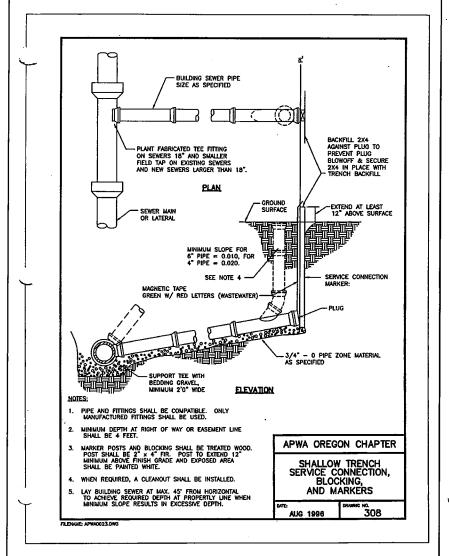
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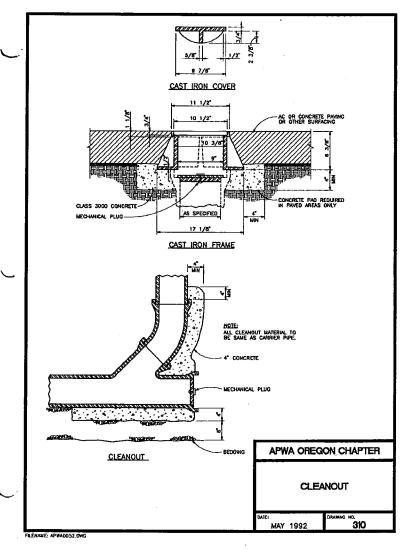
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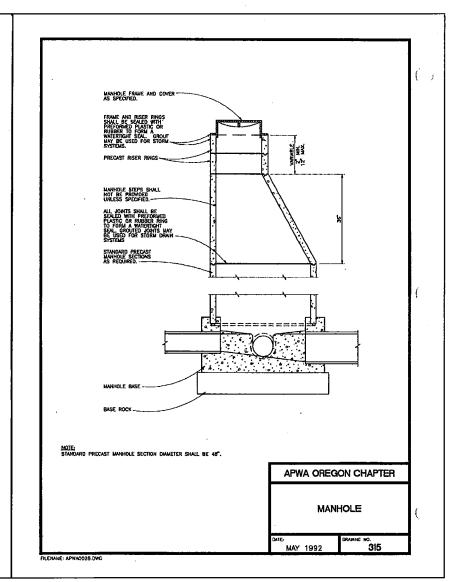
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## **Engineer's Estimate**

# "Charolais Heights East End Extension" Improvements Rough Construction Cost

2/22/2022

**Description of Improvements:** Charolais Heights a developement that over the years development has slowly added street, lots and driveways that have created a long cul-de-sac type street with a minimual trun around at the end and unimproved access to 7th street. This project improves the end of the street to current City standards and adds sewer, water, power, and fiber services for 4 additional lots and improves emergency access to 7th Street with a street right-of-way and base rock to creat a all-weather road.

**Typical Section** 

Length

25 ft. wide Paved Section

760 ft

3 in. of HMAC over

4 in. of 3/4"-0" Crushed Rock Base over

8 in. of 2"-0 Crushed Base Rock

Description	QUAN.	UNIT	UNIT PRICE	ТОТ	AL PRICE				
General									
Mobilization (7.5%)	1	LS	\$ 21,952.13	\$	21,952.13				
Temp Traffic Control	1	LS	\$ 25,000.00	\$	25,000.00				
Erosion, Sediment & Pollution Control	1	LS	\$ 5,000.00	\$	5,000.00				
Clearing/Striping	1	LS	\$ 10,000.00	\$	10,000.00				
Demo/Pulverize - Ex. AC & Conc. Removal	1	LS	\$ 10,000.00	\$	10,000.00				
Street									
Grading- Cut/Fill	4200	CY	\$ 24.00	\$	100,800.00				
Sawcut AC	140	LF	\$ 3.00	\$	420.00				
HMAC	365	Ton	\$ 135.00	\$	49,275.00				
3/4"-0" Base Rock / Shoulder Rock	300	CY	\$ 42.00		12,600.00				
2"-0" Base Rock	1550	CY	\$ 37.00	\$	57,350.00				
Waterlines									
Adjust Water Valve Boxes	6	EA	\$ 250.00		1,500.00				
Water Service	4	EA	\$ 750.00	\$	3,000.00				
New Fire Hydrant	1	EA	\$ 4,750.00	\$	4,750.00				
Sanitary									
8" 3034 PVC	275	LF	\$ 55.00		15,125.00				
4" 3034 PVC	230	LF	\$ 40.00	<del> </del>	9,200.00				
48" Manhole	2	EA	\$ 4,500.00		9,000.00				
Cleanouts	5	EA	\$ 450.00	\$	2,250.00				
Storm Drain									
New Drainage Ditching	2000	LF	\$ 3.00		6,000.00				
30" Culvert	25	LF	\$ 145.00	\$	3,625.00				
12" HDPE Storm Drain	90	LF	\$ 55.00	\$	4,950.00				
Misc. Utilities (Power, Irrigation, Fiber-optics, etc) in Common 7	Misc. Utilities (Power, Irrigation, Fiber-optics, etc) in Common Trench								
Utility Vaults	1	EA	\$ 2,500.00		2,500.00				
3" Conduit	300	LF	\$ 12.00		3,600.00				
2" Conduit	750	LF	\$ 9.00	\$	6,750.00				
Contruction Total					351,797.13				